



Finite element method (FEM1)

Lecture 10B. 2D frame element

05.2025

Examples of frame structures



Motor hang glider structure



Engine bed

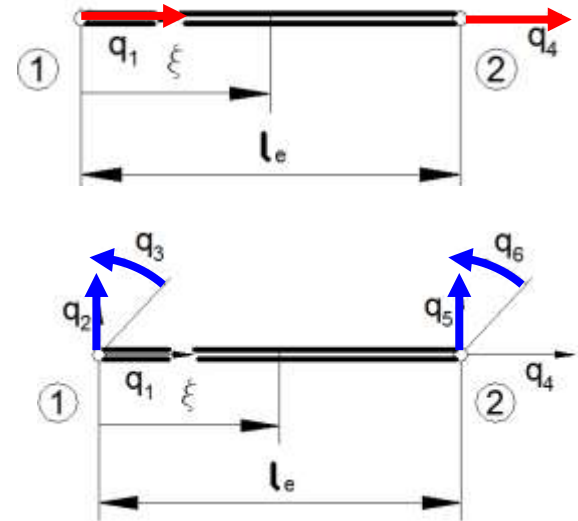
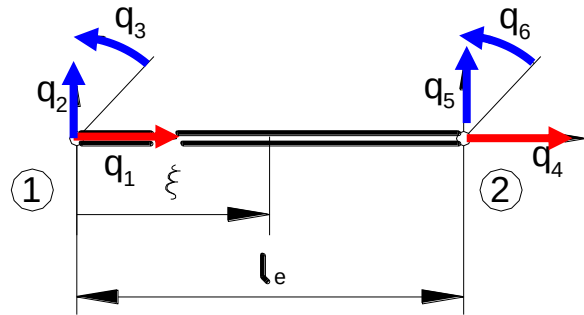


Frame construction of the hall



Bicycle frame

2D frame element in local coordinate system – local stiffness matrix

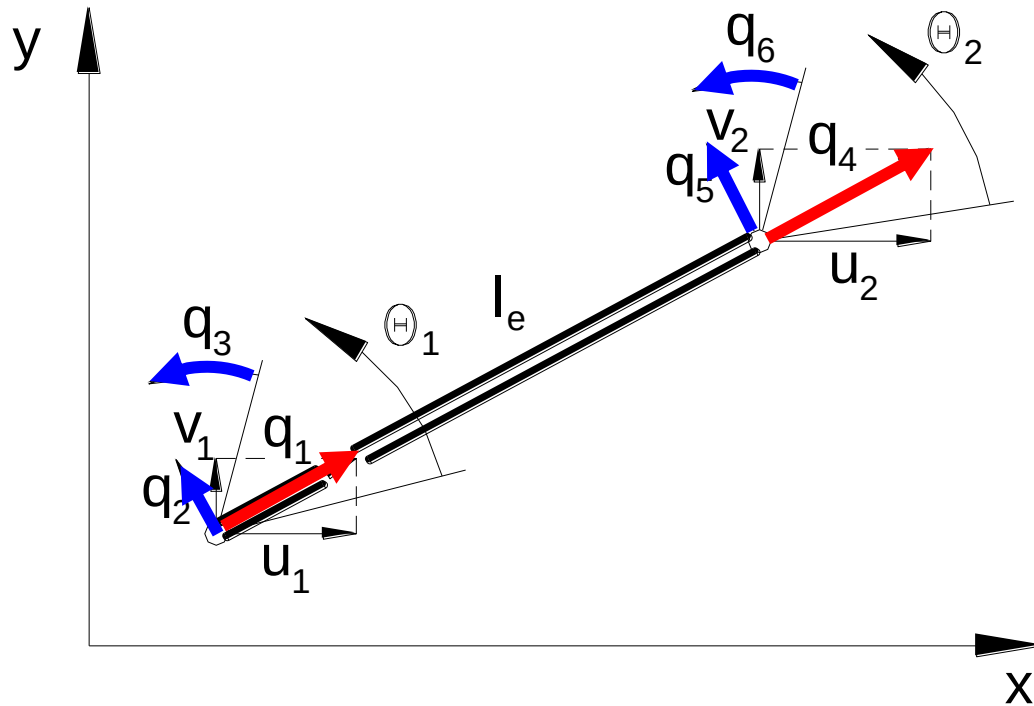


$$[k]_e = \begin{bmatrix} \frac{EA}{l_e} & 0 & 0 & -\frac{EA}{l_e} & 0 & 0 \\ 0 & \frac{12EI}{l_e^3} & \frac{6EI}{l_e^2} & 0 & -\frac{12EI}{l_e^3} & \frac{6EI}{l_e^2} \\ 0 & \frac{6EI}{l_e^2} & \frac{4EI}{l_e} & 0 & -\frac{6EI}{l_e^2} & \frac{2EI}{l_e} \\ -\frac{EA}{l_e} & 0 & 0 & \frac{EA}{l_e} & 0 & 0 \\ 0 & -\frac{12EI}{l_e^3} & -\frac{6EI}{l_e^2} & 0 & \frac{12EI}{l_e^3} & -\frac{6EI}{l_e^2} \\ 0 & \frac{6EI}{l_e^2} & \frac{2EI}{l_e} & 0 & -\frac{6EI}{l_e^2} & \frac{4EI}{l_e} \end{bmatrix}$$

$$[k]_e = \frac{EA}{l_e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[k]_e = \frac{2EI}{l_e^3} \begin{bmatrix} 6 & 3l_e & -6 & 3l_e \\ 3l_e & 2l_e^2 & -3l_e & l_e^2 \\ -6 & -3l_e & 6 & -3l_e \\ 3l_e & l_e^2 & -3l_e & 2l_e^2 \end{bmatrix}$$

2D frame element in global coordinate system – local and global nodal parameters



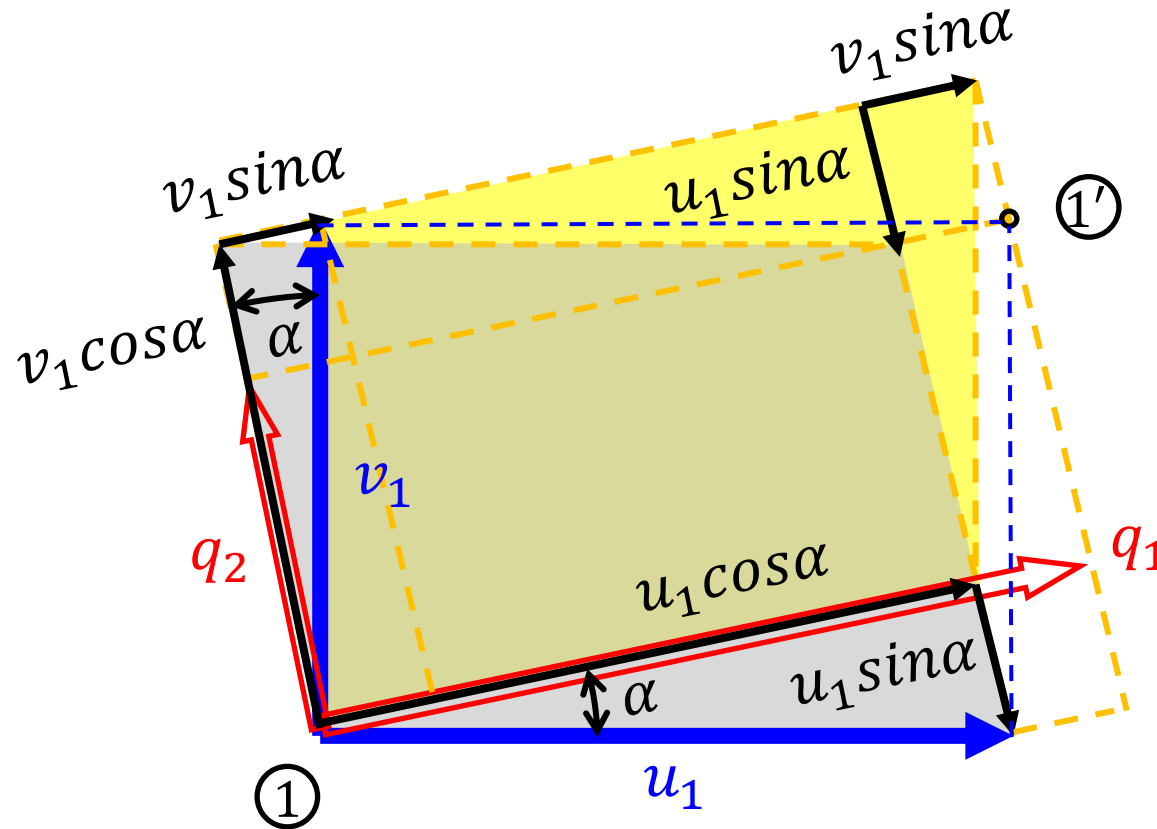
Local vector of nodal parameters:

$$\{q\}_e = \begin{Bmatrix} q_1 \\ q_2 \\ q_3 \\ q_4 \\ q_5 \\ q_6 \end{Bmatrix}$$

Global vector of nodal parameters:

$$\{q_g\}_e = \begin{Bmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \end{Bmatrix}$$

Transformation of degrees of freedom from global to local coordinate system

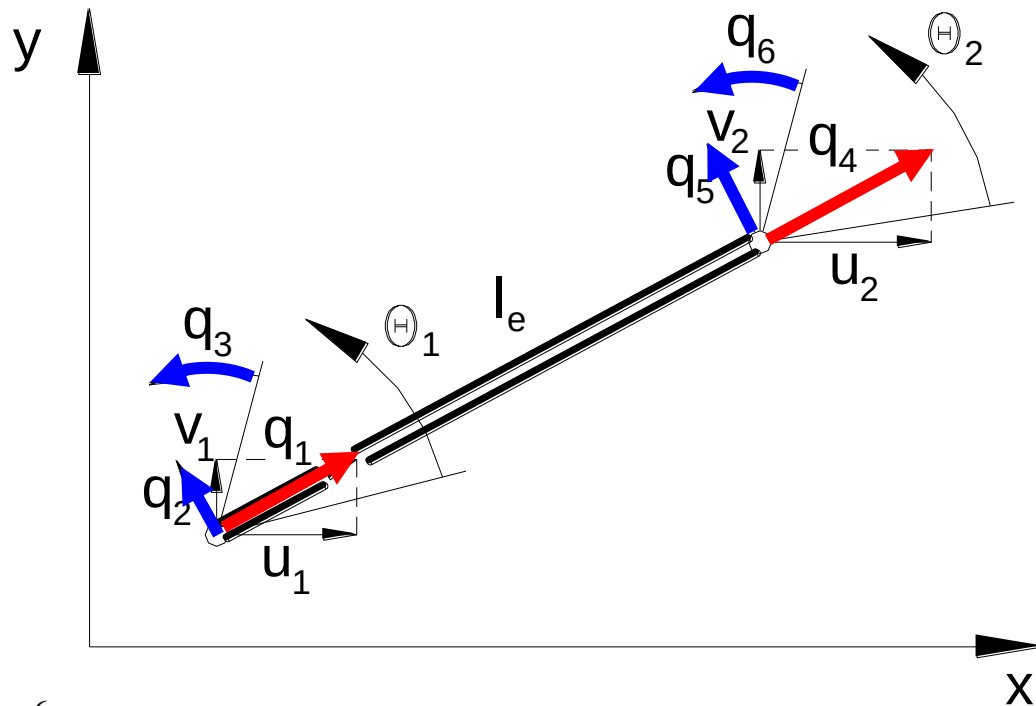


$$q_1 = u_1 \cdot \cos \alpha + v_1 \cdot \sin \alpha = c \cdot u_1 + s \cdot v_1$$

$$q_2 = -u_1 \cdot \sin \alpha + v_1 \cdot \cos \alpha = -s \cdot u_1 + c \cdot v_1$$

$$(c = \cos \alpha \quad ; \quad s = \sin \alpha)$$

2D frame element in global coordinate system – transformation matrix



Local vector of nodal parameters:

$$\{q\}_e = \begin{Bmatrix} q_1 \\ q_2 \\ q_3 \\ q_4 \\ q_5 \\ q_6 \end{Bmatrix}$$

Global vector of nodal parameters:

$$\{q_g\}_e = \begin{Bmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \end{Bmatrix}$$

$$\begin{cases} q_1 = u_1 \cos \alpha + v_1 \sin \alpha, \\ q_2 = -u_1 \sin \alpha + v_1 \cos \alpha, \\ q_3 = \theta_1. \end{cases}$$

Transformation matrix:

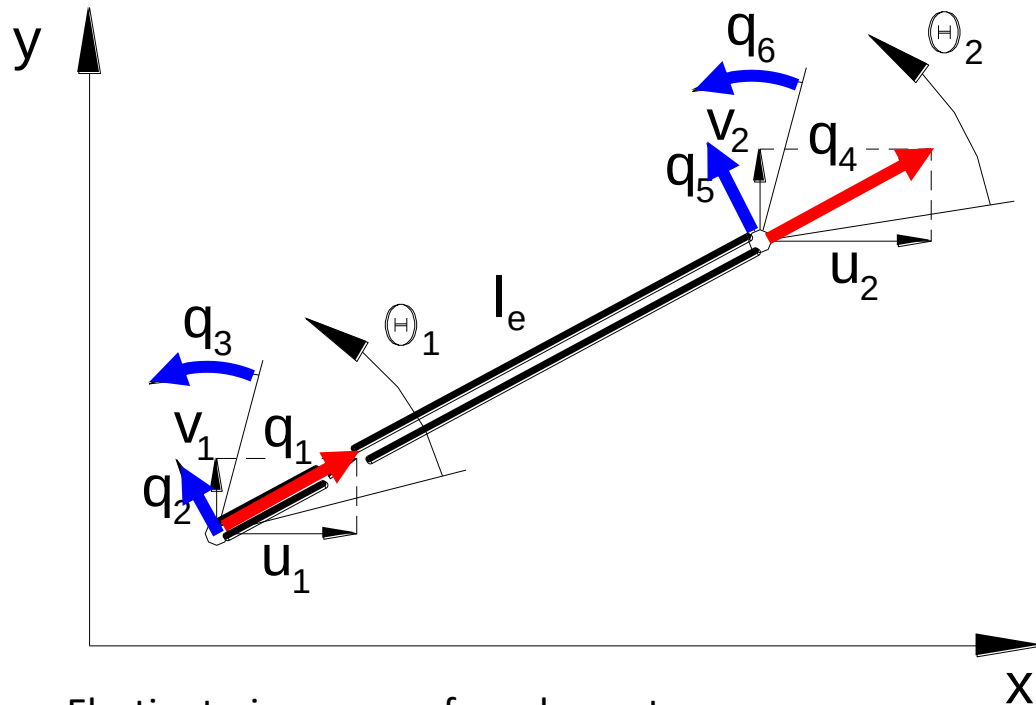
$$[T_r] = \begin{bmatrix} c & s & 0 & 0 & 0 & 0 \\ -s & c & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & c & s & 0 \\ 0 & 0 & 0 & -s & c & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$\begin{aligned} c &= \cos \alpha \\ s &= \sin \alpha \end{aligned}$$

$$[T_r]$$

$$\begin{Bmatrix} q_1 \\ q_2 \\ q_3 \\ q_4 \\ q_5 \\ q_6 \end{Bmatrix}_e = [T_r] \begin{Bmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \end{Bmatrix} = [T_r] \cdot \{q_g\}_e$$

2D frame element in global coordinate system



Local vector of nodal parameters:

$$\{q\}_e = \begin{Bmatrix} q_1 \\ q_2 \\ q_3 \\ q_4 \\ q_5 \\ q_6 \end{Bmatrix}$$

Global vector of nodal parameters:

$$\{q_g\}_e = \begin{Bmatrix} u_1 \\ v_1 \\ \theta_1 \\ u_2 \\ v_2 \\ \theta_2 \end{Bmatrix}$$

Elastic strain energy of an element:

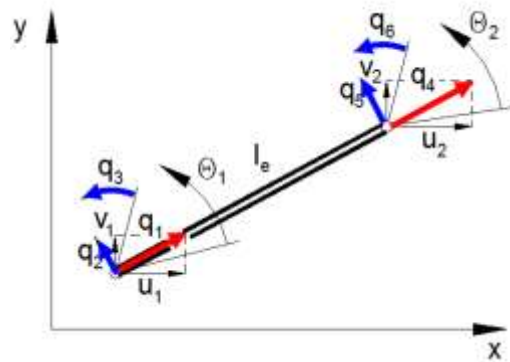
$$U_e = \frac{1}{2} [q]_e [k]_e \{q\}_e = \frac{1}{2} [q_g]_e [T_r]^T [k]_e [T_r] \{q_g\}_e,$$

$$U_e = \frac{1}{2} [q_g]_e [k^g]_e \{q_g\}_e,$$

Element stiffness matrix:

$$[k^g]_e = [T_r]^T [k]_e [T_r]$$

Global 2D frame element stiffness matrix



$$\left[k^g \right]_e = \left[T_r \right]^T \left[k \right]_e \left[T \right]$$

$\frac{EA}{l_e}$	0	0	$-\frac{EA}{l_e}$	0	0
0	$\frac{12EI}{l_e^3}$	$\frac{6EI}{l_e^2}$	0	$-\frac{12EI}{l_e^3}$	$\frac{6EI}{l_e^2}$
0	$\frac{6EI}{l_e^2}$	$\frac{4EI}{l_e}$	0	$-\frac{6EI}{l_e^2}$	$\frac{2EI}{l_e}$
$-\frac{EA}{l_e}$	0	0	$\frac{EA}{l_e}$	0	0
0	$-\frac{12EI}{l_e^3}$	$-\frac{6EI}{l_e^2}$	0	$\frac{12EI}{l_e^3}$	$-\frac{6EI}{l_e^2}$
0	$\frac{6EI}{l_e^2}$	$\frac{2EI}{l_e}$	0	$-\frac{6EI}{l_e^2}$	$\frac{4EI}{l_e}$

c	s	0	0	0	0
$-s$	c	0	0	0	0
0	0	1	0	0	0
0	0	0	c	s	0
0	0	0	$-s$	c	0
0	0	0	0	0	1

c	$-s$	0	0	0	0
s	c	0	0	0	0
0	0	1	0	0	0
0	0	0	c	$-s$	0
0	0	0	s	c	0
0	0	0	0	0	1

